

APPENDIX E: GEOTECHNICAL REPORT



Ground Investigation Report

The Harlington, Fleet

Prepared For







Report Approval

Report Author(s)	Signature	Date
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Revision – Requested by	Nature of Revision	Date



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Trading Terms

Unless specifically stated within the tender/quotation or unless identified within the introduction to this report it is confirmed that this report has been compiled wholly in accord with Impact Geotechnical Ltd's terms of engagement. This report is provided for sole use by the Client and is confidential to them. No responsibility whatsoever for the contents of the report will be accepted to anyone other than the Client.

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Context

This report is written in the context of an agreed scope of work between Impact Geotechnical Ltd and the Client and should not be used in a different context. In light of additional information becoming available, improved practices and changes in legislation amendment or re-interpretation of the report in whole or part may be necessary after its original submission.

Professional Interpretation

The recommendations made and opinions expressed in the report are based on the conditions revealed by the site works together with an assessment of the data from the insitu and laboratory testing or in respect of the desktop reports. No responsibility can be accepted for conditions that have not been revealed by the research, site works and testing.

The Client is advised that the conditions observed on site by Impact Geotechnical Ltd at the time of any site survey may be subject to change. Certain indicators of the presence of hazardous substances may have been latent at the time of the most recent site reconnaissance and they may subsequently have become evident. It is not possible to assess areas which are inaccessible or where access is not granted and IGL accept no liability for risks subsequently identified therein.

The conceptual model, Risk assessment and sampling regime has been formulated in accordance with current UK guidance at time of production based upon the relevant information gained from Phase 1 and Phase 2 investigations. While the model and assessment offer opinions and interpretations of these guidelines, the comments made are for guidance only and no liability can be accepted for their accuracy. It is possible that aspects of Geo-environmental reports may need to be altered following consultation with the statutory regulatory bodies to suit planning requirements.

Intrusive Field Operations

The data collected through direct operations in the production of this report has been so obtained, unless directly otherwise stated, in accordance with current UK guidance, law or accepted industry practice, including but not limited to: BS.5930: 1990 Code of Practice for Site Investigations (Amendment 3: 2015), & BS.10175: 2011 + A1: 2013 Investigations into Potentially Contaminated Sites. Exact exploratory locations will depend upon access conditions, site use and plant capability, IGL do not accept liability for issues arising from material identified between or outside of the area of exploratory locations.

Laboratory Testing

Unless stated otherwise within the text, all geotechnical and material laboratory tests have been performed in accordance with the relevant British Standard Documents. Laboratory testing for contaminated land assessment is completed under the UKAS / MCERTS accreditation schemes, unless identified as otherwise in the report.

Human Health Risk Assessment Criteria

The Environment Agency has undertaken revision of the Soil Guideline Values (SGVs) which are partially complete. Where standards are available using the "new" approach, these have been utilised for correlative purposes. Where standards have not yet been revised, guidance following the "old" approach has been utilised. Please note that upon release of the remaining guidelines, the standards contained within this report may be subject to change. In addition, the second edition of the LQM CIEH guidance has now been released and will be utilised in favour of previously published guideline values.

Third Parties

The findings and opinions conveyed in this report are based on information obtained from a variety of sources, including that from previous Site investigations and chemical testing laboratories. IGL has assumed that such information is correct. IGL cannot and does not guarantee the authenticity or reliability of the information it has relied upon and can accept no responsibility for inaccuracies with the data supplied by other parties.

The accuracy of the historical map extracts supplied can not be guaranteed and it should be noted that different conditions may have existed between mapping sheet editions. Therefore, there can be no certainty that all areas of contamination have been identified during the Phase 1 investigation.

Definitions

Reference to the word "contamination" in this report does not relate to the statutory definition of contaminated land under 1990 Environmental Protection Act unless otherwise stated. The definition used in this report is: "Land that contains substances that, when present in sufficient quantities or concentrations, are likely to cause harm, directly or indirectly, to man, to the environment, or on occasion to other targets" (NATO CCMS, 1985).

IGL 2019



1.0 INTRODUCTION

Impact Geotechnical Ltd (IGL) were instructed by Cooper and Withycombe (the Client) (Q19.134, dated: July 2019), to carry out a Ground Investigation at The Harlington, 236 Fleet Road, Fleet, GU51 4BY (hereafter referred to as the "site").

This report relates to the potential structural alterations and/or construction of new extensions the existing building. The purpose of the investigation was to establish preliminary information for the client, in order to determine the most cost-effective approach to the proposed construction works.

The investigation incorporated the excavation of four hand excavated trial pits to expose the foundation details to various sections of the building and the construction of three window sample boreholes to a maximum depth of 5.00mbgl in order to provide stratigraphy and geotechnical parameters.

The aims of this report are to provide an outline Ground Model of the proposed development area, and to inform the detailed design of temporary and permanent works associated with the planned construction.

2.0 SITE LAYOUT

The existing site is comprised of 'The Harlington' building, which is theatre/ entertainment venue. The building is centred on an approximate national grid reference of SU 80641 54034.

The building comprises of central section which forms a concert venue, with several extensions of varying age and construction. A basement structure is located in the northeast corner. Fleet library is also attached to the southwest corner of the building.

The building is surrounded in hardstanding in all directions, with car parking facilitates located to the west and east, with Harlington Way to the north. South of the site leads to the main Fleet Road, which is the main town high street.

The site is generally flat, however a gradual slope towards the south is noted.

3.0 PHYSICAL SETTING

3.1 Geology

The Geology of Britain Viewer (BGS, 2019) indicates the site is underlain by the Camberley Sand Formation, however superficial deposits of the Surrey Hills Gravel Member are found locally.

<u>Surrey Hills Gravel Member</u> – Comprising of Sand and Gravel, with the gravel portion comprised of flint, lower greensand and sparse quartz and quartzite.

<u>Camberley Sand Formation</u> – The Camberley Sand Formation comprises a fairly uniform sequence of yellow brown, sparsely to moderately glauconitic silty fine-grained Sand and sandy Silt, with some ironstone concretions and white sandstone. Sporadic flint gravel or gravel beds occur near the base of the unit and thin beds of pale grey Clay occur intermittently throughout.



4.0 FIELDWORKS

The following intrusive works were carried out over the over two days; Friday 2nd and Monday 5th August 2019 supervised by an Engineering Geologist from IGL. The SI was undertaken in accordance with the scope of works agreed with our Client and in relation to statutory guidance including BS5930: 1999 Code of Practice for Site Investigations (Amendment 3: 2015) and BS10175: 2011+A1: 2013 Investigation of Potentially Contaminated Sites: Code of Practice.

- Prior to any excavations taking place a Cable Avoidance Tool (CAT) was used to check for the position of any underlying electrical services. In addition, starter pits were excavated to 1.00 meters below ground level (mbgl) to clear test locations prior to any further drilling commencing.
- Four hand excavated trial pits (TP1-TP4) were excavated against various building elevations to expose the structural foundation details. Once exposed, detailed hand sketches were completed, together with photographic evidence and representative soils samples taken.
- Three windowless sample boreholes (WS1-WS3) were constructed to a maximum depth of 5.45mbgl using a tracked windowless sample rig. The recovered soils from each borehole and groundwater conditions were logged, with representative samples recovered to allow subsequent testing.
- Upon completion, exploratory holes were backfilled using arising materials and the surface finish reinstated to match existing.

The site layout plan indicating the position of the test location is provided in Appendix A, with photographs taken during the investigation in Appendix C.

5.0 GROUND CONDITIONS

5.1 Soils

The following table summarises the strata conditions encountered during the intrusive works:

Stratum	Depth Range Encountered	Detail Description
Surface Cover	GL – max.0.15m	Taramacdam in WS1, TP2 and TP3 to 0.08-0.10mbgl. Within TP4/WS3 brick pavers over sharp Sand to 0.15mbgl.
		The surface cover in TP1 and WS2 was described as Made Ground composed of dark brown gravelly silty Sand to 0.45-0.50mbgl.
Made Ground	0.08-0.15 to 1.40m	Within WS1, a brownish grey very sandy Gravel was encountered below the tarmac surface to a maximum depth of 0.50mbgl.
		Within TP2 and TP3, 'Type 1' fill was recorded to a depth of 0.22m (TP2) and 0.55mbgl (TP3) Below this TP2 a thin concrete slab was encountered to 0.30mbgl, laid over a 100mm diameter plastic drainage pipe. Pea shingle surrounded the pipe, to a maximum depth of 0.70mbgl. Below this a black silty/ clayey Sand was encountered to a maximum excavated depth of 1.40mbgl.



		Within WS2, re-worked soils described as a brown mottled orangish brown, locally reddish brown slightly gravelly Sand was encountered
		between 0.50-0.90mbgl.
		Within TP4/ WS3, below the brick pavers/ sharp Sand, 'Type 1' fill was encountered to 0.33mbgl, overlying a dark brown slightly gravelly silty/ clayey Sand to a maximum depth of 1.10mbgl.
Camberley Sand Formation (CSF)	0.50-1.10m to 2.50-2.60m	Described as a medium dense brown mottled orangish brown, locally reddish brown / greenish grey, slightly gravelly silty, locally clayey fine to medium Sand. The gravel portion as described as sub-angular to sub-rounded flint.
Camberley Sand Formation (CSF)	2.50-2.60 to 5.45m	Medium dense becoming dense yellowish brown mottled orangish brown and greenish grey, glauconitic silty fine to medium Sand.

Please refer to the stratigraphic logs contained within Appendix B for a more detailed description.

5.2 Groundwater

No groundwater was encountered during the drilling of either of the exploratory boreholes.

5.3 Visual and Olfactory Observations of Contamination

With the exception of anthropogenic materials encountered within the Made Ground soils, no visual or olfactory evidence of soil or groundwater contamination was noted during the investigation works.

5.4 Existing Foundations

The construction form of the existing foundations was explored within TP1-TP4. The table below summarises the findings of these investigations.

Trial Pit Ref:	Foundation Type	Depth to top of concrete (mm)	Base depth & (Thickness) (mm)	Projection (mm)	Founding Soil
TP1 A-A'	Concrete Strip	450	1150 (700)	150	Mottled Sand (CSF)
TP2 A-A'	Basement wall	500	Proven to 1400 (900)	50	n/a
ΤΡ3 Α-Α'	Concrete Strip	460	1400 (940)	80	Mottled Sand (CSF)
TP3 B-B'	Concrete Strip	460	1400 (940)	150	Mottled Sand (CSF)
TP4 A-A' Concrete Pad		240	1140 (900)	580 (A'A) x 550	Mottled Sand (CSF)

6.0 Insitu Testing

6.1 Standard Penetration Testing

Standard Penetration testing was completed throughout the drilling of WS1-WS3 at 1.00m centres to 5.00mbgl. This form of testing is completed using a 63.5kg drop hammer weight, over a 750mm drop, measuring the blow



counts for six, 75mm increments. The first two values are recorded as seating blows, with the remaining four values, added together to provide an 'N-value'.



The graph below summarises the results of this testing, by conversion to equivalent SPT-N values.

Testing through the upper Camberley Sand Formation to 2.50-2.60mbgl, provided SPT N-values of N=14-29, which are indicative of generally medium dense soils.

Below 2.50-2.60mbgl, SPT N-values are seen to largely increase, with value of N=15-50 recorded, which are indicative of medium dense becoming dense/ very dense soils.

7.0 LABORATORY TESTING

7.1 Geotechnical Testing

7.1.1 Particle Size Distribution (PSD)

In total, three disturbed samples of the underlying soils were submitted for Particle Size Distribution (PSD) testing by wet sieve; classification testing to determine the percentage, range and grain sizes of soil types. The table below provides a summary of the testing:

Sampla Bafi	Grain Size Percentage												
Sample Ker.	Gravel (%)	Sand (%)	Silt (%)	Clay (%)									
TP1 1.20m	8	70	2	2									
TP3 1.40m	15	68	1	7									
WS3 1.50m	0	71	2	7									
WS1 2.00m	0	84	1	6									
WS3 3.00m	0	89	1	1									

Results of testing confirm that of the logging engineers' descriptions, indicating the soils to be primarily a Sand, gravelly at shallow depths, and becoming less silty/clayey with depth.

7.1.2 Sulphate and pH Analysis

Five samples between depths of 1.00-1.45m and 3.00-3.45mbgl were submitted for determination of pH and Water-Soluble Sulphate concentration.

Water soluble sulphate concentrations were found to range from 14mg/l to 73mg/l, with pH levels ranging from 5.2 to 7.2.

Full laboratory test results can be found in Appendix D.

8.0 DISCUSSION OF GROUND CONDITIONS

8.1 Soil Engineering Properties

The purpose of this investigation was to provide a summary of the foundation construction to the existing building(s) as well as the underlying soil conditions, in relation to potential structural alterations and/or construction of new extensions to the existing building as well as the possible replacement of the current structure. The information gained by this investigation will provide the client with preliminary information, in order to determine the most cost-effective approach to the proposed construction works.

Stratigraphic records indicate a variable depth of Made Ground (below an initial surface of Taramacdam or brick pavers in WS1, TP2, TP3 and TP4/WS3), ranging between maximum depths of 0.50-1.40mbgl. In general, this was described as either 'Type 1' fill material, or a re-worked dark brown or brown silty/ clayey Sand.

The initial natural soils were described as a medium dense brown mottled orangish brown, locally reddish brown / greenish grey, slightly gravelly silty, locally clayey fine to medium Sand. This material was encountered to a maximum depth of 2.50-2.60mbgl and is considered representative of the Camberley Sand Formation.

The exposed foundations to the existing building(s) were observed to be founded within this material. Traditional concrete strip foundations were observed in TP1 and TP2, sited at a depth of 1.15-1.40mbgl, with concrete pad foundations in TP4, sited a depth of 1.14mbgl. The full depth of the foundation details in TP2 could not be established; we understand that there is a basement structure within this area, and as such foundations are likely to be in the region of 3.00-4.00mbgl.



On review of the insitu strength testing completed within this material in WS1-WS3, this material is considered medium density, with SPT N-values of N=14-29 recorded.

The soils conditions at depth below 2.50-2.60mbgl were recorded as a yellow brown mottled orangish brown and greenish grey, glauconitic silty fine to medium Sand. This material was encountered to a maximum drilled depth of 5.45mbgl in all locations and is also considered representative of the Camberley Sand Formation.

Further insitu testing within this material suggests a medium dense, becoming dense/very dense consistency, with SPT N-values of N=15-50 recorded.

No groundwater was encountered within any of the exploratory holes during the investigation.

8.2 Shallow Foundations

The Made Ground soils encountered across the site would not be considered suitable as a bearing stratum; soils of this origin are frequently present in a weak and variable condition such that unacceptable settlement would be anticipated even under light loading intensities.

The soils at the existing foundation depths of 1.14-1.40mbgl are described as a medium dense brown mottled orangish brown, locally reddish brown / greenish grey, slightly gravelly silty, locally clayey fine to medium Sand. On the basis of insitu testing completed within this material a safe bearing capacity of 125-150kN/m² is considered achievable at this depth.

For new foundations, we would suggest that foundation depths should match that of existing foundations (1.20m>). The upper portion of the Camberley Sand Formation was noted to be locally clayey and as such excavations should be checked during the construction phase for any variability in soil conditions. If encountered, any soft spots encountered should be removed and replaced with suitable fill material foundation extended to a greater depth, or bridged, where possible.

8.3 Floor Slabs

The floor slab design will depend on the final foundation designs and is dependent on the underlying materials, including bearing capacity and the presence of any cohesive or Made Ground Soils.

Where Made Ground thicknesses extend beyond 600mm, it is recommended that a suspended floor is utilised for any new structure, unless a subbase can be designed, and constructed to a suitable degree of compaction, beneath the new floor slab. This should be designed by a structural engineer; the completion of integrity testing of the subbase should be considered, prior to forming the new concrete slab.

8.4 Excavations and Groundwater

It is likely that excavations of less than 1.20mbgl will require support to their faces due to the variable/ granular nature of the Made Ground. Should excavations be taken below this, adequate support should be provided in order to satisfy statutory safety regulations.

Groundwater was not encountered during the investigation, however groundwater levels are dependent upon seasonal variations and levels may change after periods of heavy rain of prolonged drought.



If groundwater is identified within any of the excavations during the construction phase, then it should be dealt with appropriately and removed using good engineering practices.

The design of any temporary retaining structures to support excavation faces should be made assuming the following moderately conservative parameters:

Material	Effective angle of friction (Ø')	Effective Cohesion; c' (kPa)	Bulk Density kN/m ³
Made Ground	28-30	0	17-18
Camberley Sand Formation	30-32	0	18-19

8.5 Aggressive Chemical Environment to Concrete

Sulphate concentrations were found to range from 14mg/l to 73mg/l, with pH levels ranging from 5.2 to 7.2, suggesting that a design class of DS-1 and a sub class of AC-3z should be adopted for buried concrete structures within these soils, (Reference made to current BRE SD1 Guidelines) based on the soils tested.

APPENDICES

Appendix A – Site Investigation Plan Appendix B – Stratigraphic Logs Appendix C – Photographs Appendix D – Laboratory Certificates









Site Investigation Plan





Notes:

- Do not scale from this drawing.
 All dimensions must be checked
- on site prior to commencement of work.
- Where applicable this drawing is to be read in conjunction with other consultants drawings.
- This drawing is the copyright of Impact Geotechnical Ltd.

Drawing Title:

Site Investigation Plan

Project Reference:

P19.114

The Harlington, Fleet

Revision: 0 Drawn by: SG Scale: Not to Scale

Site Name:



N A





Stratigraphic Logs



						Bore	ehole	e Log	Borehole No. WS1 Sheet 1 of 1				
Projec	t Name:	The Harling	ton, Fle	et	Proje	ect No		Co-ords	Hole Type				
<u> </u>	-				P19	.114		n/a	WLS Scale				
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Hand excavated pit to 1.00mbgl. No groundwater encountered. SPT's completed at 1.00m centres. End of windowless sample at 5.45mbgl. Backfilled with arisings on completion.

Project Name: The Harlington, Fleet Project No P19.114 Co-ords n/a Hole Type WLS Location: Fleet, Hampshire Level Scale NTS Client: Cooper & Withycombe Date(s) 05/08/2019 Logged By 05/08/2019 SG Well Sample and Insitu Testing Wetrix Depth(s) Type Results Depth (m) Level MOD Legend AOD Stratum Description 0.80 D 0.50 0.50 Osrower dark brown graveliv strikes Oncore the brown graveliv osrower dark brown graveliv stratum Description Image: Concrete Construct and concrete. MADE GROUND) Grass over dark brown graveliv ub-angular to sub-counded flint, brick and concrete. 0.80 D 0.50 0.50 Osrower dark brown graveliv stratum Description Image: Concrete.							Bore	ehole	e Log	Borehole No. WS2 Sheet 1 of 1	
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Hand excavated pit to 1.00mbgl. No groundwater encountered. SPT's completed at 1.00m centres. End of windowless sample at 5.45mbgl. Backfilled with arisings on completion.

Project Name: The Harlington, Fleet Project No P19.114 Co-ords n/a Hole Type Wils Location: Fleet, Hampshire Level Scale n/a NTS Client: Cooper & Withycombe Date(s) Logged By 05/08/2019 SG Well Sample and Insitu Testing Water Depth(s) Type Results Depth (m) Level (mADE GROUND) Philos brown sandy fine to coarse, sub-angular to sub-rounded Gravel of Limetoria (rover 1) Fill 1.00 D 1.20 SPT D N=14 (2,4/4,3,3,4) 1.10 Medium dense brown, locally reddish brown and pale brown, locally reddish brown greenish grey slighty gravelly sity, locally clayer fine to medium SAND. Gravel is fine to coarse, sub-angular to sub-rounded fine t				÷			Bore	ehole	e Log	Borehole No. WS3 Sheet 1 of 1				
Location: Fleet, Hampshire P19.114 n/a WLS Location: Fleet, Hampshire Level Scale n/a NTS Client: Cooper & Withycombe Date(s) Logged By Well Sample and Insitu Testing Results Depth (m) Level (m) Stratum Description Well Water Depth(s) Type Results Depth 0.15 Legend AOD Stratum Description 0.15 Image: Cooper & Withycombe 0.15 0.33 Image: Cooper & Withycombe MADE GROUND) Parkish brown sandy fine to coarse, sub-angular to sub-rounded Gravel of image: Sub-angular to sub-rounded finit and brick. 1.00 D SPT N=14 (2,4/4,3,3,4) 1.10 Medium dense brown, nocally reddish brown and pale brown, locally reddish brown/ greenish grey slightly gravelly slivy, locally clayer fine to medium SAD. Gravel is fine to coarse, sub-angular to sub-rounded flint. (CAMEERLEY SAND FORMATION) Medium dense brown mottled orangish brown and pale brown, locally reddish brown/ greenish grey slightly gravelly slivy, locally clayer fine to medium SAD. Gravel is fine to coarse, sub-angular to sub-rounded flint. (CAMEERLEY SAND FORMATION)	Proje	ct Name:	The Harling	ton, Fle	et	Proje	ect No		Co-ords	Hole Type				
Location: Fleet, Hampshire Detect n/a NTS Client: Cooper & Withycombe Date(s) Logged By Well Sample and Insitu Testing Depth (m) Level (m) Legend Stratum Description Well Water Depth(s) Type Results Depth (m) Level (m) Legend Stratum Description Well Strikes Depth(s) Type N=14 (2,4/4,3,3,4) 0.15 Kinke GRO UND) Dark brown alightly gravelly silty/clayey fine to medium Sand. Gravel is fine to coarse, sub-angular to sub-rounded finat and brick. Imach Location D D N=14 (2,4/4,3,3,4) 1.10 Medium dense brown mottled orangish brown and pale brown, locally readish brown and pale brown, locally readish brown and pale brown, locally readish brown/ greenish gree slightly gravelly silty, locally clayey fine to sub-rounded fint. (CAMBERLEY SAND FORMATION) 2.00 D N=20 (2,3/3,5,6,6) 2.50 D						P19	.114	 	n/a	WLS Scale				
Depth (s) Logged By 05/08/2019 Well Sample and Insitu Testing Water Depth(s) Type Results Depth (m) Level (m Level (m Level (m Stratum Description Well Water Depth(s) Type Results 0.15 Birick payers ave sharpe sand. 0.33 0.15 0.33 0.33 0.15 MADE GROUND) Pinkish brown sand; fine to coarse, angular to sub-rounded Gravel of limestane (TYPE 1) Fill With/clayer fine to be-ounded Gravel of limestane (TYPE 1) Fill With/clayer fine to medium Sand. Gravel Is fine to coarse, sub-angular to sub-rounded fint and birk. 1.00 D N=14 (2,4/4,3,3,4) 1.10 Medium dense brown mottled orangish brown and pale brown, locally reddish brown/ greenish grey slightly gravelly slity, locally clayer fine to medium SAND. Gravel Is fine to coarse, sub-angular to sub-rounded fint. (CAMBERLEY SAND FORMATION) 2.00 D N=20 (2,3/3,5,6,6) 2.50 East	Locati	ion:	Fleet, Hamp	pshire					n/a	NTS				
Cooper & Withycombe O5/08/2019 SG Sample and Insitu Testing Level (m Level (m Level (m Stratum Description Well Water Depth(s) Type Results Depth (m) Level (m Legend Stratum Description Well Strikes Depth(s) Type Results 0.15 Level (m Legend Stratum Description Well Image: Colspan="4">Image: Colspan="4">Stratum Description Well Image: Colspan="4">Image: Colspan="4">Stratum Description Well Image: Colspan="4">Image: Colspan="4">Stratum Description Image: Colspan="4">Image: Colspan= 400000 Stratum Description	Cliont		Canar 8. V						Date(s)	Logged By				
Sample and Insitu Testing Results Depth (m) Level (m) Legend Stratum Description Well Water Strikes Depth(s) Type Results 0.15 Birick pavers ove sharpe sand. (MADE GROUND) Pinkish brown sandy fine to coarse, angular to sub-rounded Gravel of Limestane (Type 1) Eill (MADE GROUND) Pinkish brown sandy fine to coarse, angular to sub-rounded Gravel of Limestane (Type 1) Eill (MADE GROUND) Pinkish brown sandy fine to coarse, angular to sub-rounded Gravel of Limestane (Type 1) Eill 1.00 D N=14 (2,4/4,3,3,4) 1.10 Medium dense brown mottled orangish brown and pale brown, locally reddish brown/ greenish grey slightly gravelly silty, locally clayey fine to medium SAND. Gravel is fine to coarse, sub-angular to sub-rounded flint. 2.00 D N=20 (2,3/3,5,6,6) N=20 (2,3/3,5,6,6) N=20 (2,3/3,5,6,6) 2.50 D N=20 (2,3/3,5,6,6) 2.50 N=20 (2,3/3,5,6,6) N=40		.: 	Cooper & v	Vitriycon	nbe			05	5/08/2019	SG				
Weil Water Strikes Depth(s) Type Incluits (m) AOD) Cegenal Structure procession Strikes Depth(s) Type (m) (m) AOD) AOD) Structure procession View Incluits (m) (m) AOD) Structure procession Structure procession View (mADE GROUND) Pinkish brown sandy fine to coarse, angular to sub-rounded Gravel of limestone (TYPE 11) Fill (mADE GROUND) Dark brown slightly gravelly slitty/clayey fine to medium Sand. Gravel is fine to coarse, sub-angular to sub-rounded flint and brick. 1.00 D SPT N=14 (2,4/4,3,3,4) 1.10 1.20-1.65 D D Medium dense brown mottled orangish brown and pale brown, locally redish brown/ greenish grey slightly gravelly slight, locally clayey fine to medium SAND. Gravel is fine to coarse, sub-angular to sub-rounded flint. (CAMBERLEY SAND FORMATION) 2.00 2.00 SPT N=20 (2,3/3,5,6,6) Image: sliphtly gravelly SAND FORMATION)	م/۸/	Sample	e and Insitu Te	sting	Results	Depth	Level (m	Legend	Strat	um Description				
1.00 D 1.00 D 1.20 SPT 1.20 SPT 1.20 SPT 1.20-1.65 D 0 N=14 (2,4/4,3,3,4) 1.10 Medium dense brown mottled orangish brown/ greenish grey slightly gravelly silty, locally clayey fine to medium SAND. Gravel is fine to coarse, sub-angular to sub-rounded flint and brick. 2.00 D 2.50 D	VVCII	Strikes	Depth(s)	Туре	hesuits	(m)	AOD)	Legena	50.00					
3.00 3.03,345 D D N=31 (4,5/7,7,8,9) Medium dense becoming dense, yellowish brown mottled orangish brown and greenish grey glaconitic, sith (net or mottled orangish brown and greenish grey glaconitic, sith (net or mottled orangish brown and greenish grey glaconitic, sith (net orangish brown grey glaconitic, sith) grey glaconitic, sith (net orangish brown grey gla			$ \begin{array}{c} 1.00\\ 1.20\\ 1.20-1.65\\\\ 2.00\\ 2.00\\ 2.00-2.45\\\\ 2.50\\\\ 3.00\\ 3.0$	D SPT D D SPT D D SPT D D SPT D D SPT D	N=14 (2,4/4,3,3,4) N=20 (2,3/3,5,6,6) N=31 (4,5/7,7,8,9) N=38 (3,5/7,9,10,12) N=40 (4,5/9,9,11,11)	0.15 0.33 1.10 2.50 5.45			Birick pavers ove sha (MADE GROUND) Pink coarse, angular to sub- Limestone. ('TYPE 1') F (MADE GRO UND) Da silty/clayey fine to m to coarse, sub-angular brick. Medium dense brow and pale brown, loca greenish grey slightly clayey fine to mediur coarse, sub-angular t (CAMBERLEY SAND F Medium dense becor brown mottled orang grey glauconitic, silty (CAMBERLEY SAND F End of bor	rpe sand. ish brown sandy fine to rounded Gravel of ill irk brown slightly gravelly edium Sand. Gravel is fine ir to sub-rounded flint and n mottled orangish brown lly reddish brown/ gravelly silty, locally n SAND. Gravel is fine to o sub-rounded flint. ORMATION) ming dense, yellowish gish brown and greenish fine to medium SAND. ORMATION) ehole at 5.45mbgl				

Hand excavated pit to 1.20mbgl. No groundwater encountered. SPT's completed at 1.00m centres. End of windowless sample at 5.45mbgl. Backfilled with arisings on completion.



Remarks

Hand excavated pit to 1.20mbgl to expose existing foundation construction. No groundwater encountered. Backfilled with arisings and previous sufrace conditions reinstated.





Remarks

Hand excavated pit to 1.45mbgl to expose existing foundation construction. No groundwater encountered. Backfilled with arisings and previous sufrace conditions reinstated.



Remarks

Hand excavated pit to 1.45mbgl to expose existing foundation construction. No groundwater encountered. Backfilled with arisings and previous sufrace conditions reinstated.



Hand excavated pit to 1.40mbgl to expose existing foundation construction. No groundwater encountered. Backfilled with arisings and previous sufrace conditions reinstated. WS3 completed through base of trial pit to 5.45mbgl - see WS3 log for full details.











WS1 3.00-4.00mbgl





9. 10. 12. 11. **Investigation Photographs** 9. TP1 10. TP2 **IMPACT** GEOTECHNICAL TP2 Project Ref: P19.114 11. 12. WS2 Site Name: The Harlington, Fleet



16.



20. TP3





28. WS3 1.20-2.00mbgl



WS3 4.00-5.00mbgl 32. TP1 and WS1 reinstatement







Laboratory Certificates



	K	3)		PARTIC	LE SIZE	ION		Job Ref	/Dit No	26971								
	ita Na		/	The Herlington	Floot							Borenoie			IP	1			
0						Oliant						Sample I	NO.		-				
Pr	ojecti	NO.		P19-11	ŧ	Client			Impa	ci Geol	ecnnical		р		1.20		m		
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	Tes	t Metho	d	BS1377:Part 2:	1990, clau	se 9.0						Projec	t started						
	_											Date	tested		21/08/2019				
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	-	1.18	3	91 91						_	Remarks								
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		0.15	5	43															
		0.06	3	22										0.			va d		
5	Q					K4 Soil	s Lab	orat	ory					Check	ed and A	Approv	vea		
Unit 8, Olds Close, Watford, Herts, WD18 9RU Initials: J.P												J.P							
	AS				En	nail: jamo Tel: 01	es@k 923 7	4soi 7112	IS.CO 38	om			Date:	Date: 22/08/2019					
25	519	Арр	roved S	ignatories: K.Ph	aure (Tech	.Mgr) J.Pha	ure (La	b.Mgr)					MSF-5-R3					

	K	H SOILS		SIZ	IZE DISTRIBUTION Job Ref Borehole										ef					26971					$\overline{\neg}$							
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	0.11			Ora	ngish bro	wn slig	htly m	nottle	ed gr	ey g	rave	lly cl	aye	y SA	AND	(gra	avel i	s fm	с	Sa	mple	Type	e.		-				<u>ר</u>			
	Soll L	Jescripti	ion		0	0	and s	ub-a	angul	lar to	sub	-rou	nde	d)						Sa	ample	s ree	ceive	ed	+		C	5/08	/201	9		-
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	-				SILT						S	AND)							GRA	VEL							DC				
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Unit 8, Olds Close, Watford, Herts, WD18 9RU												I	Initia	als:					J.	Ρ												
	KAS						C 10	T	. jai el: (019	23 7	711	288	3	/11								I	Date	:			2	22/08	/201	9	
2	519	Арр	roved S	ignato	ries: K.Ph	aure (1	Tech.	Mgr)	J.Pł	naure	e (La	ıb.M	gr)											MSF-5-R3								

	K	1		Р	ARTIC	LE SIZE	DIS	TRI	BUT	ION			Job Ref	-		269	71		
		SOILS											Borehol	e/Pit No.		WS	1		
S	ite Na	me		The Harlington, F	leet			1					Sample	No.	_	-			
Pr	oject N	No.		P19-114		Client			Impa	ct Geot	echnical		Depth T	ор		2.00		m	
											Depth Base			ase		-		m	
	Soil D	escriptio	on	Orang	ish brown	slightly mot	tled g	rey silf	ty clay	ey SAI	ND		Sample	Туре		D			
													Sample	es received		05/08/2019			
	Test	Method	1	BS1377:Part 2: 1	90, clause 9.0 Pro						Proje	ct started		08/08/2019					
													Dat	e tested		21/08/2	2019		
	_	CLAY	Fine	SILT	Coorco	Fino	S	AND		Coorco	Fino		GRAVEL	Coorno	COBBLES	BOU	LDERS		
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			Sie	ving		Sedime	ntatio	n											
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		63		100							Sand					83.	9		
		50 37.5		100 100	_					-	Fines <(0.063	3mm		-	16.	1		
		28		100									-			-			
		20 14		100 100	_					-	Grading D100	g An	alysis	mm	-				
		10		100							D60			mm		0.19	95		
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		3.35		100							Uniform	ity C	oefficient						
		2		100						-	Curvatu	re C	oefficient						
		0.6		100						1	Remark	S							
		0.425	5	99 96	_					-	Preparatio	n and	testing in acco	rdance with BS13	77 unless not	ed below			
		0.212	2	69															
		0.15 0.063	8	33 16															
G	þ					K4 Soils	s Lat	oorat	orv						Check	ed and A	Approv	red	
- ()	4)			Unit	8, Olds	Close, V	Vatfo	rd, H	lerts	, WD	18 9RU			Initia	Initials: J.P			I.P	
	Email: james@k4soils.com							Date	Date: 22/08/2019)19							
25	19	Appr	oved S	ignatories: K.Phau	ire (Tech.l	Mgr) J.Phau	ire (La	ab.Mgr	.)							MSF-5-F	२३		

	K	1)	Р	ARTIC	LE SIZE	DIS	TRIB	UT	ION			Job	Ref				26	971	0.10	
		soils											Bor	ehole	/Pit No.	-		W	'S3		
S	ite Na	me		The Harlington, F	leet								Sar	nple I	No.	_			-		
Pr	oject N	No.		P19-114		Client		l	mpac	t Geot	echnical		Dej	oth To	р			1.50			m
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	Soil D	escriptio	on	Orang	gish brown	mottled blu	uish gr	ey silty	claye	ey SAN	D		Sample Type						D		
													Samples received				05/08/2019				
	Test	Method	1	BS1377:Part 2: 1	990, clause 9.0						Project started				08/08/2019						
														Date	tested			21/08	3/2019	9	
	_	CLAY	Fine	SILT	Coorso	Eino	S	AND		`ooroo	Fin	_	GRA\	/EL	Coorno	COBBLE	ES	В	DULDE	RS	
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		0.425	5	98							Preparatio	on and	testing in	accord	lance with BS13	77 unless	noted	below			
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	L	0.063	}	29												~	ocl	d =:: '	A	A	3
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Shaun Gilbrook Impact Geotechnical Ltd 26 Anmore Road Denmead Hants PO7 6NP



DETS Ltd Unit 1 Rose Lane Industrial Estate Rose Lane Lenham Heath Kent ME17 2JN t: 01622 850410

DETS Report No: 19-11367

Site Reference:	The Harlington, Fleet
Project / Job Ref:	P19.114
Order No:	None Supplied
Sample Receipt Date:	08/08/2019
Sample Scheduled Date:	08/08/2019
Report Issue Number:	1
Reporting Date:	14/08/2019

Authorised by:

Dave Ashworth Technical Manager

Opinions and interpretations are outside the laboratory's scope of ISO 17025 accreditation. This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This certificate shall not be reproduced except in full, without the prior written approval of the laboratory.



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Soil Analysis Certificate						
DETS Report No: 19-11367	Date Sampled	None Supplied				
Impact Geotechnical Ltd	Time Sampled	None Supplied				
Site Reference: The Harlington, Fleet	TP / BH No	WS1	WS1	WS2	WS3	WS3
Project / Job Ref: P19.114	Additional Refs	None Supplied				
Order No: None Supplied	Depth (m)	1.00 - 1.45	3.00 - 3.45	2.00 - 2.45	1.20 - 1.65	2.00 - 2.45
Reporting Date: 14/08/2019	DETS Sample No	426588	426589	426591	426592	426593

Determinand	Unit	RL	Accreditation					
рН	pH Units	N/a	MCERTS	5.2	6.4	7.2	7.0	5.5
W/S Sulphate as SO_4 (2:1)	mg/l	< 10	MCERTS	73	27	27	14	14
W/S Sulphate as SO_4 (2:1)	g/l	< 0.01	MCERTS	0.07	0.03	0.03	0.01	0.01

Analytical results are expressed on a dry weight basis where samples are assisted-dried at less than 30^oC Subcontracted analysis (S)



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Soil Analysis Certificate - Sample Descriptions
DETS Report No: 19-11367
Impact Geotechnical Ltd
Site Reference: The Harlington, Fleet
Project / Job Ref: P19.114
Order No: None Supplied
Reporting Date: 14/08/2019

DETS Sample No	TP / BH No	Additional Refs	Depth (m)	Moisture Content (%)	Sample Matrix Description
^ 426588	WS1	None Supplied	1.00 - 1.45	7.1	Brown sandy clay with stones
^ 426589	WS1	None Supplied	3.00 - 3.45	8.1	Brown sandy clay
^ 426591	WS2	None Supplied	2.00 - 2.45	12.3	Brown loamy sand
^ 426592	WS3	None Supplied	1.20 - 1.65	11.8	Brown sandy clay
^ 426593	WS3	None Supplied	2.00 - 2.45	11.4	Brown sandy clay

Moisture content is part of procedure E003 & is not an accredited test

Insufficient Sample ^{I/S}

Unsuitable Sample ^{U/S}

^ no sampling date provided; unable to confirm if samples are within acceptable holding times



DETS Ltd Unit 1, Rose Lane Industrial Estate Rose Lane Lenham Heath Maidstone Kent ME17 2JN Tel : 01622 850410



oil Analysis Certificate - Methodology & Miscellaneous Information	
ETS Report No: 19-11367	
npact Geotechnical Ltd	
te Reference: The Harlington, Fleet	
roject / Job Ref: P19.114	
rder No: None Supplied	
eporting Date: 14/08/2019	

Matrix	Analysed	Determinand	Brief Method Description	Method
Soil		Poron Water Soluble	Determination of water coluble baren in coil by 2:1 bet water extract followed by ICD OES	E010
Soll			Determination of PTEX by beadenase CC MS	E012
Soil	AR		Determination of cations in soil by agua regia digestion followed by ICP OES	E001 E002
Soil	D	Chloride - Water Soluble (2:1)	Determination of chloride by extraction with water & analysed by ion chromatography	E002
Soil			Determination of hexavalent chromium in soil by extraction in water then by acidification, addition of	E007
	AR		1,5 diphenylcarbazide followed by colorimetry	EUTO
Soil	AR	Cyanide - Complex	Determination of complex cyanide by distillation followed by colorimetry	E015
Soll	AR	Cyanide - Free	Determination of free cyanide by distillation followed by colorimetry	E015
Soll		Cyanide - Total	Determination of total cyanide by distillation followed by colorimetry	E015
Soil		Diosol Papeo Organics (C10 C24)	Determination of hoveno/acotono ovtractable bydrocarbons by CC ELD	EUTT EOO4
	AN	Dieser Kange Organics (CTO - CZ4)	Determination of electrical conductivity by addition of saturated calcium sulphate followed by	L004
Soil	AR	Electrical Conductivity	electrometric measurement	E022
Soil	AR	Electrical Conductivity	Determination of electrical conductivity by addition of water followed by electrometric measurement	E023
Soil	D	Elemental Sulphur	Determination of elemental sulphur by solvent extraction followed by GC-MS	E020
Soil	AR	EPH (C10 – C40)	Determination of acetone/hexane extractable hydrocarbons by GC-FID	E004
Soll	AR		Determination of acetone/hexane extractable hydrocarbons by GC-FID	E004
Soil	AR	C12-C16, C16-C21, C21-C40)	headspace GC-MS	E004
Soil	D	Fluoride - Water Soluble	Determination of Fluoride by extraction with water & analysed by ion chromatography	E009
Soil		EOC (Fraction Organic Carbon)	Determination of fraction of organic carbon by oxidising with potassium dichromate followed by	F010
			titration with iron (II) sulphate	
Soil	D	Loss on Ignition @ 450oC	Determination of loss on ignition in soil by gravimetrically with the sample being ignited in a muffle furnace	E019
Soil	D	Magnesium - Water Soluble	Determination of water soluble magnesium by extraction with water followed by ICP-OES	E025
Soil	D	Metals	Determination of metals by aqua-regia digestion followed by ICP-OES	E002
Soil	AR	Mineral Oil (C10 - C40)	Determination of hexane/acetone extractable hydrocarbons by GC-FID fractionating with SPE cartridge	E004
Soil	AR	Moisture Content	Moisture content; determined gravimetrically	E003
Soil	D	Nitrate - Water Soluble (2:1)	Determination of nitrate by extraction with water & analysed by ion chromatography	E009
Soil	D	Organic Matter	Determination of organic matter by oxidising with potassium dichromate followed by titration with iron (II) sulphate	E010
Soil	AR	PAH - Speciated (EPA 16)	Determination of PAH compounds by extraction in acetone and hexane followed by GC-MS with the use of surrogate and internal standards	E005
Soil	AR	PCB - 7 Congeners	Determination of PCB by extraction with acetone and hexane followed by GC-MS	E008
Soil	D	Petroleum Ether Extract (PEE)	Gravimetrically determined through extraction with petroleum ether	E011
Soil	AR	pH	Determination of pH by addition of water followed by electrometric measurement	E007
Soil	AR	Phenols - Total (monohydric)	Determination of phenols by distillation followed by colorimetry	E021
Soil	D	Phosphate - Water Soluble (2:1)	Determination of phosphate by extraction with water & analysed by ion chromatography	E009
Soil	D	Sulphate (as SO4) - Total	Determination of total sulphate by extraction with 10% HCI followed by ICP-OES	E013
Soll	D	Sulphate (as SO4) - Water Soluble (2:1)	Determination of sulphate by extraction with water & analysed by ion chromatography	E009
50II		Sulphate (as SO4) - Water Soluble (2:1)	Determination of water soluble sulphate by extraction with water followed by ICP-UES	EU14 EO10
5011 Soll		Sulphur Total	Determination of sulphice by distillation followed by COOffficity Determination of total sulphur by extraction with aqua-regia followed by LCP OES	EUIO FA24
501	. –		Determination of semi-volatile organic compounds by extraction in acetone and hexane followed by GC-	
Soil	AR	SVOC	MS	E006
Soil	AR	Thiocyanate (as SCN)	Determination of thiocyanate by extraction in caustic soda followed by acidification followed by addition of ferric nitrate followed by colorimetry	E017
Soil	D	Toluene Extractable Matter (TEM)	Gravimetrically determined through extraction with toluene	E011
Soil	D	Total Organic Carbon (TOC)	Determination of organic matter by oxidising with potassium dichromate followed by titration with iron (11) subpate	E010
Soil	AR	TPH CWG (ali: C5- C6, C6-C8, C8-C10, C10-C12, C12-C16, C16-C21, C21-C34, aro: C5-C7, C7-C8, C8-C10, C10-C12, C12-C16, C16-C21, C21-C35)	Determination of hexane/acetone extractable hydrocarbons by GC-FID fractionating with SPE cartridge for C8 to C35. C5 to C8 by headspace GC-MS	E004
Soil	AR	TPH LQM (ali: C5-C6, C6-C8, C8-C10, C10 C12, C12-C16, C16-C35, C35-C44, aro: C5-C7, C7-C8, C8-C10, C10-C12, C12- C16, C16-C21, C21-C35, C35-C44)	Determination of hexane/acetone extractable hydrocarbons by GC-FID fractionating with SPE cartridge for C8 to C44. C5 to C8 by headspace GC-MS	E004
Soll		VUUS VIDU (CK CO 0 CO C10)	Determination of hydrocarbons $C6$ $C8$ by boadspace CC MS 8 $C9$ $C10$ by $C0$ ELD.	
2011	AK	VPH (LO-LÖ & LÖ-LIU)	Determination of hydrocardons co-co by headspace GC-NIS & Co-CTU by GC-FTD	EUUI

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APPENDIX F:

OUTLINE DRAWINGS FOR IMPROVEMENT WORKS:

- ESSENTIAL AND NECESSARY
- DESIRABLE





















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